Design of Structures (3150612)

B.E. Semester 5 (Civil Engineering)



Shantilal Shah Engineering College Shantilal Shah Engineering College, Bhavnagar



Directorate of Technical Education Gandhinagar, Gujarat

Shantilal Shah Engineering College, Bhavnagar

Certificate

| This is to certify that Mr./Ms | |
|---|------------------|
| Enrollment No | of B.E. |
| Semester 5 th Civil Engineering of this institute (GTU Code: | <u>043</u>) has |
| satisfactorily completed the tutorial work for the subject I | Design of |
| Structures (3150612) for the academic year | • |

Place: <u>Bhavnagar</u>

Date: _____

Name and Sign of Faculty member

Head of the Department

Important Instructions for Students

- [1] Use A-4 size blank pages to prepare design report.
- [2] The report shall have following format (lay out)

| | Main Heading | Page No |
|--------------------------|--|--|
| Margin for Bullets | Sub heading Design calculation/write up | Margi n to write refere nces |

- [3] Use only front side of page for write up as well as for sketches and detailing.
- [4] Mark 1¹/₂" margins on both left and right. Use left margin to show bullets and right margin to show references like clause of code/page etc.
- [5] Start new design problem on new page.
- [6] Heading and sub heading shall be distinct than write up.
- [7] Support the calculation/s with neat sketches wherever required.
- [8] Attach A3 size sheets to furnish design detailing (Use AutoCAD Drawing for at-least one design problem).
- [9] All design detailing shall be strictly as per relevant IS standards.
- [10] Spiral bound is preferred as it is more convenient for reading.
- [11] Student shall bring required IS codes and other references in class and tutorial hours as listed below.
- [12] IS 456: 2000, IS 800: 2007, IS 875 (Part I to V), SP: 6 (Part-1)] all the Codes includes latest Amendment), steel table

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(Progressive Assessment Sheet)

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PART A: TUTORIAL – 1-6 (Reinforced Concrete Structures)

TUTORIAL – 1

INTRODUCTION

| | COs | LEVEL |
|--|------|-------|
| 1. Define the aim of structural design. | 1 | R |
| 2. List out various methods of structural design. | 1 | R |
| 3. Write down the difference between Limit State Method (LSM) and Working Stress Method (WSM). | 1 | R |
| 4. Define characteristic cube compressive strength of concrete (fck) and characteristic strength of steel (fy). | 1, 2 | R |
| 5. Define tensile strength of steel (fct). | 1, 2 | R |
| 6. Write advantage and disadvantages of RCC. Why steel is used as reinforcement in RCC? | 1, 2 | R |
| 7. List out the combination of loads as per IS-1893. | 1, 2 | R |
| 8. Calculate effective span for (a) Simply supported beam and slab (b) continuous beam and slab (c) cantilever beam as per IS 456(2000) | 1, 2 | E |
| 9. Define basic values of span to effective depth ratios for Cantilever beam, simply supported beam and Continuous beam (a) for spans up to 10m. (b) Spans above 10m. | 1, 2 | R |
| 10. Explain difference between column and strut. | 1 | U |
| 11. Explain maximum diameter of bar used in slab. | 1, 2 | U |
| 12. Define nominal cover to reinforcement in slab, footing, column and beam in moderate condition. | 1, 2 | R |
| 13. Define max. and min. % of reinforcement required in beam. | 1, 2 | R |
| 14. Define (1) Balance section (2) Under reinforced section (3) Over reinforced section | 1, 2 | R |
| 15. Explain types of Load. List out various IS codes used for calculation of loads. | 1, 2 | R |

TUTORIAL – 2 BEAMS

| | | COs | LEVEL |
|-----|---|------|-------|
| Sir | ngly Reinforced Beam | | |
| 1. | For a limiting section 300mm x 600mm gross Determine the following if M-20 | 3,5 | Ε |
| | conc. Mix and fy = 415 N/mm^2 steel is used (i) Max. Compressive stress and | | |
| | max. Tensile stress (ii) Lever arm (iii) Total compression (iv) Total tension (v) | | |
| | Limiting moment (vi) Area of tensile steel. | | |
| 2. | A RCC beam rectangular in section 230mm x 450mm effective is singly | 3,5 | Е |
| | reinforced by 4 no. of 16 mm dia. Bars of fe-415 grade steel and M-20 grade of | | |
| | concrete. Determine moment of resistance of section. | | |
| 3. | Determine moment of resistance for a beam 230mm x 350mm overall size and | 3,5 | Ε |
| | effective cover 40 mm. The beam is reinforced with 5 no. 16mm dia. Bars take | | |
| | fck=20 N/mm ² , fy=415 N/mm ² . | | |
| 4. | A singly R.C. beam effective section 300mm x 600mm, provide with $3-20\Phi + 3-$ | 3,5 | Е |
| | 16 Φ at effective cover of 50 mm is simply supported 4.50 m. span. Use fck=20 | | |
| | N/mm ² , fy=415 N/mm ² . Evaluate safe load (L.L.) on beam. | | |
| 5. | Design singly R.C. balance section for factored moment 225 kN-m. Use Fe-415 | 4, 5 | С |
| | steel and M-20 grade of concrete. Take width to effective depth ratio for the beam | | |
| | 0.7. | | |
| 6. | Design singly R.C. beam having width 230 mm, simply supported with effective | 4, 5 | С |
| | span of 4.0m. it is loaded with a U.D.L. of 15 kN/m excluding self weight. Use | | |
| | M-20 grade concrete and Fe-415 steel. Check the beam for max. and min. steel | | |
| | and deflection. | | |
| 7. | Explain Stress -Strain diagram for singly Reinforced concrete beam. Derive | 1, 2 | R |
| | equation of Moment of Resistant for balanced section. | | |
| 8. | Determine the moment of resistance of a beam section 230 mm X 600mm | 3, 5 | Е |
| | effective depth reinforced with 3- nos. 25 mm diameter bars. M20 grade concrete | | |
| | and Fe- 250 steel reinforcement is used. Also find out the moment of resistance if | | |
| | the materials are M-20 and Fe -415. Comment on the answer. | | |
| Do | ubly Reinforce Beam | | |
| 1. | Explain necessity of doubly R.C. beam | 3, 4 | U |
| 2. | Explain stress diagram for doubly R.C. beam. | 3, 4 | U |
| 3. | A doubly reinforced beam of 300 mm x 600 mm overall is reinforced with 5-16 | 3, 5 | Ε |
| | Φ bars as compression reinforcement and 5-20 Φ bars as tensile reinforcement. | | |

| Effective cover on both sides is 50mm, grade of concrete M-25 and steel Fe-415. | | |
|---|------|---|
| Determine (i) types of section (ii) Moment resistance capacity of the section. | | |
| 4. A rectangular beam of size 200mm x 350mm effective depth is subjected to a | 3, 5 | Е |
| factored moment of 150 kN-m. Determine the reinforcement for flexure. The | | |
| effective cover for the tensile and compression steel are 50 mm. The materials are | | |
| M-25 grade concrete and HYSD reinforcement of grade Fe-415. | | |
| 5. Evaluate the area of tensile and compression reinforcement required for a | 3, 5 | Ε |
| rectangular beam of size 230mm x 500mm effective for the factored moment of | | |
| 325 kNm. The effective cover for the tensile and compression steel are 50 mm. | | |
| The materials are M-20 grade concrete and HYSD reinforcement of grade Fe- | | |
| 415. | | |
| <u>T Beam</u> | | |
| 1. A RCC T-beam section reinforce for tension has the following data, | 3, 5 | E |
| Flange width = 1600 mm | | |
| Thickness of Flange = 125 mm | | |
| Effective depth = 700 mm | | |
| Width of $rib = 325 mm$ | | |
| Determine the limiting moment of resistance of the section. Take M-20 concrete and | | |
| Fe-415 steel. | | |
| 2. Calculate limiting value of M.R. of T-beam with the following data, | 3, 5 | Ε |
| Flange width = 1900 mm | | |
| Depth of Flange = 130 mm | | |
| Effective depth of beam = 550 mm | | |
| Width of web = 300 mm | | |
| Take M-20 concrete and Fe-415 steel 4-no. of 25 mm dia. | | |
| 3. Determine M.R. of T-beam with the following data, | 3, 5 | Е |
| Flange width = 1500 mm | | |
| Depth of Flange = 115 mm | | |
| Effective depth of beam = 425 mm | | |
| Width of web = 300 mm | | |
| Take M-20 concrete and Fe-415 steel 5-no. of 20 mm dia. | | |
| 4. Determine M.R. of T-beam with the following data, | 3, 5 | Ε |
| Flange width = 1500 mm, Depth of Flange = 115 mm | | |
| Effective depth of beam = 425 mm | | |
| Width of web = 300 mm | | |
| Take M-20 concrete and Fe-415 steel 5-no. of 20 mm dia. | | |

TUTORIAL – 3 SLABS

| | | COs | LEVEL |
|----|---|---------|-------|
| 1. | Explain various types of slabs with sketch. | 3, 4 | U |
| 2. | Design and detail simply supported slab on 300 mm wide brick masonry for a clear | 4, 5 | С |
| | room size 4 m x 10 m. use material grade M-20 and Fe-415. Take live load as 3.5 | | |
| | kN/m^2 and floor finish as 1 kN/m^2 . | | |
| 3. | Design for the slab of the hall of school building 10×8 m with provision of two | 4, 5 | С |
| | intermediate beams 300 \times 500 mm at a clear distance of 3.5 m. the slab is resting | | |
| | on four walls of 300 mm thick and carrying live load of 3.5 $\mathrm{kN/m^2}$. Show the | | |
| | details of reinforcement for the slab by sketch. Use M-20 and Fe 415. | | |
| | | | |
| 4. | A one-way continuous slab of 150 mm thickness resting on 300 mm wide brick | 4, 5 | С |
| | masonry supports spaced at 4 m c/c. considering live load as 2.5 kN/m^2 and floor | | |
| | finish as 1 kN/m ² . Design and detail slab for span moment and support moment. | | |
| | Assume four spans of the slab. Take M-20 grade of concrete and Fe 415 grade of | | |
| | steel. | | |
| 5. | The 1 m wide single flight R.C.C. stair case is to be provided for a height of 2.6 m | 3, 4, 5 | Ε |
| | in a residential building. Staircase is supported at top and bottom risers by beams | | |
| | 300 mm wide. Waist slab is 180mm thick. Riser 200 mm and tread is 300 mm. | | |
| | Evaluate effective span, design load, reinforcement in waist slab. Prepare of | | |
| | sketch use M20 and Fe 415. | | |
| 6. | Design a simply supported slab of 3×4.5 m effective span supported on 300 mm | 4, 5 | С |
| | thick walls on all four sides. Assume live load 3 kN/m^2 and floor finish load 0.5 | | |
| | kN/m^2 . Use M20 and Fe 415. Corners are not held down. | | |
| 7. | Design and detail Reinforced Concrete slab for a room 6m x 5m. The slab is to be | 4, 5 | С |
| | cast monolithically over beams with corners held down. The width of supporting | | |
| | beams 230 mm. Slab carries superimposed load of 3 kN/m ² . Use M20 and Fe415. | | |

TUTORIAL-4 COLUMN

| | COs | LEVEL |
|--|------|-------|
| 1. Classify various types of columns based on its (a) Shape (b) Bracing system (c) | 3, 4 | U |
| Lateral and longitudinal reinforcement (d) Effective Length of column (e) Types | | |
| of loading. | | |
| 2. State the assumptions in design of compression member. | 3 | R |
| 3. A reinforced shot column of 400 mm x 450 mm in cross section is to carry an | 3, 5 | E |
| axial factored load of 1680 kN calculate the area of steel required and the spacing | | |
| of 8 mm dia. Lateral ties. Use concrete M-20 and steel Fe-415. Give detail sketch | | |
| of the section. | | |
| 4. A short RCC rectangular column of 300 mm x 450 mm is reinforced with 6no. Of | 3, 5 | E |
| longitudinal bars (4no. of 25 mm dia. And 2no. of 20 mm dia) determine load | | |
| carrying capacity of the column if M-25 mix and fe-415 steel is used also design | | |
| the column for lateral ties. Check the column for minimum eccentricity. | | |
| Unsupported length of column is 3.20 m. Give detail sketch of the section. | | |
| 5. Design rectangular RC column for an axial load 1500 kN use M-20 concrete and | 4, 5 | С |
| Fe-415 steel also check for eccentricity, unsupported length of column is 3.50 m. | | |
| 6. Design a short circular column for an axial compressive factored load of 950kN. | 4, 5 | С |
| The grade of concrete M-25 steels Fe-415 it is to be provided with minimum | | |
| reinforcement sketch the detail. | | |

TUTORIAL – 5 FOOTING

| | | | | COs | LEVEL |
|--|---|---|--|------------|-------|
| 1. | A rectangular colu | mn of size 230 x 600 mm is lo | aded with 900 kN characteristic | 3, 4, 5 | E |
| | load. The safe bear | ring capacity of soil is 200 KN | N/m^2 . Determine the dimension | n | |
| | of the footing for the | ne following cases. | | | |
| | a. If footing is squ | are. | | | |
| | b. If the footing ha | as equal projection in all four si | ides. | | |
| | c. If the dimension | n parallel to the shorter side of | column is restricted to 2 m. | | |
| | d. If the dimension | n parallel to the longer side is r | estricted to 2.5 m. | | |
| 2. | Design an isolated | I square pad footing for a squ | are column 300 x 300 mm for | 4,5 | С |
| | axial load of 1700 | kN. Use concrete grade M-2 | 25 and fe-415 steel grade. Take | • | |
| | safe bearing capaci | ty of soil- 140 kN/m ² . Also dr | aw neat sketch. | | |
| 3. | Design a rectangul | ar isolated sloped footing for | a column of size 230 x 600 mm | 4,5 | C |
| | carrying an axial cl | haracteristic load of 1800 kN a | and reinforced with 8 nos 20 dia | | |
| | Bars in M-25 gra | de concrete. The allowable b | pearing pressure on soil is 250 |) | |
| | kN/m^2 . The mate | erials for footing are grade | e M-25 concrete and HYSE | | |
| | reinforcement of g | rade Fe-415. | | | |
| 4. | 4. Determine the plan dimensions of a combined footing for two axially loaded | | | 1 3, 4, 5 | E |
| | columns with following data if (1) Width is not restricted, considering 1 m | | | ı | |
| | projection from C1 (2) Width is restricted to 2.3 m. Assume self weight of | | | f | |
| | footing is 15% of axial loads. | | | | |
| | Columns | C1 | C2 | | |
| | Туре | Interior | Interior | | |
| | Size | 400mm x 400 mm | 400 x 400 mm | | |
| | Р | 1000 Kn | 1200 | | |
| | Spacing | 3 m c/c from C1 to C2 | | | |
| | SBC/ABP150 kN/m2 at 1.6 m depth | | | | |
| | 5. Design a combine rectangular footing for 1200 kN and 1800 kN column loads | | | 5 | |
| spaced at 4 m. centre to centre. Consider following data for the design. | | | | C | |
| • | • Size of each column 450 mm x 450 mm | | | 4,5 | |
| • | $SBC = 250 \text{ kN/m}^2$ | | | | |
| • | Use M20 concrete a | and Fe-415 grade steel | | | |
| | | | | | |

TUTORIAL – 6

BOND, DEVELOPMENT LENGTH AND SHEAR REINFORCEMENT

| | | COs | LEVEL |
|----|---|---------|-------|
| 1. | Explain anchoring of reinforcement. | 3 | U |
| 2. | A simply supported R. C. C. beam with clear span of 5m, support width 230 | 3, 4, 5 | С |
| | mm, size of beam 230 wide and 420mm deep, tension bars as 4nos. of 16mm | | |
| | dia. bars and clear cover of 25mm. If it is loaded by an all inclusive factored udl | | |
| | of 60kN/m, Design the shear reinforcement near support only using 2 legged | | |
| | 6mm. mild steel stirrups. | | |
| 3. | Explain various types of shear reinforcement with sketch. | 3 | U |

PART B: TUTORIAL – 7 To 10 (Steel Structures)

TUTORIAL-7

INTRODUCTION & TENSION MEMBER

| | COs | LEVEL |
|--|----------|-------|
| 1. Write down various advantages and disadvantages of steel structure. | 3, 4 | R |
| 2. List out series of rolled steel (i) I-sections, (ii) Channel section, and (iii) Angle | 3,4 | R |
| sections. | | |
| 3. Explain the advantages of bolted connections over riveted or welded connections. | 3,4 | U |
| 4. Elaborate the effect of shear leg in tension member with necessary sketch. | 3, 4 | С |
| 5. Determine the design tensile strength of the plate 200mm x 12mm with the holes for | 3,5 | Ε |
| 16mm diameter bolts as shown in fig.1 Steel use is of Fe-415 grade quality. | | |
| $\leftarrow \begin{bmatrix} & & & & \\ & & & & \\ & & & & \\ & & & &$ | | |
| Fig1 | | |
| 6. A single unequal-leg angle 90x60x6mm is connected to a 10mm thick at the ends | 3,5 | E |
| with 5 no. of bolts of 16 mm diameter bolts to transfer tensile force. Determine the | ; | |
| design tensile strength of the angle. (i) if gusseted plate (G.P.) is connected to 90mm | L | |
| angle(ii) if G.P. is connected to 60mm angle | | |
| 7. Design and detail a connection for a truss member 2-ISA60x60x8mm connected | 4,5 | С |
| back-to-back on both the sides of a 10mm thick gusset plate using M20 bolts of | • | |
| property class 4.6 grade. The axial tensile factored load in the member is 150kN. | | |
| 8. Determine the tensile strength of a roof truss diagonal $100 \ge 75 \ge 6$ mm having fy = | 3, 5 | E |
| 250 MPa connected to gusset plate by 4 mm welds of 140 mm long at top and 310 | J | |
| mm long at bottom. The longer edge of 100 mm was connected to plate of 8mm | <u>.</u> | |
| thickness. | | |
| 9. Design a lap joint and butt joint between two plates each of width 120mm. If the | 4, 5 | C |
| thickness of one plate is 16mm and other is 12 mm. the joint has to transfer a design | | |
| load of 160 kN. Plates are of Fe-410 grade. Calculate the efficiency of the joint. | | |
| Assume 4.6 grade bolts. | | |
| 10. Design a single angle section for a tension member of a roof truss to carry a factored | 4,5 | C |
| tensile force of 225 kN. Take length of member 3m. use M-20mm shop bolt of grade | : | |
| 4.6. | | |

TUTORIAL – 8

COMPRESSION MEMBER, LACING AND BATTENING

| | | COs | LEVEL |
|-----|---|------|-------|
| 1. | Explain different end conditions of columns with their effective length. | 3, 4 | U |
| 2. | Distinguish between behavior of short and long compression members. | 3, 4 | Ν |
| 3. | Explain IS 800 recommendations for compression member in trusses. | 3, 4 | U |
| 4. | Design axial load capacity of the column ISHB 300@ 577 N/m if the length of | 4, 5 | С |
| | column is 3m and its both ends pinned. | | |
| 5. | <u>Calculate</u> the compressive resistance of a compound column consisting of | 3, 5 | Ε |
| | ISHB 300 with one cover plate 350 x 20 mm on each flange and having a length | | |
| | of 5 m. assume that bottom of the column is fixed and top is pinned, $fy = 250$ | | |
| | MPa | | |
| 6. | Determine axial compressive load carrying capacity of a 2.3m long single angle | 3, 5 | E |
| | strut ISA75x50x8mm. The longer leg is connected to the gusset plate with two | | |
| | bolts at each end. Assume hinged condition. | | |
| 7. | <u>Calculate</u> compressive strength of 2 ISA 80 x 80 x 8 mm placed on either side | 3, 5 | Ε |
| | of gusset plate 8 mm thick with effective held in position at both ends but | | |
| | restrained against rotation at one end. The length of member is 3 m and fy is 250 | | |
| | MPa. | | |
| 8. | Design a single angle strut connected to the gusset plate to carry 180 kN | 4, 5 | С |
| | factored load. The length of the strut between centre to centre inter section is 3 | | |
| | m. | | |
| 9. | Explain laced and battened columns with sketch. | 3, 4 | U |
| 10. | Design a laced column with two channels back to back of length 10m to carry | 4, 5 | С |
| | an axial factored load of 1400 kN. The column may be assumed to have | | |
| | restrained in position but not in direction at both ends (hinged ends). | | |
| 11. | Design a column to carry an axial factored load of 1200 kN. The actual length | 4, 5 | С |
| | of column is 6m with both ends effectively held in position and restrained | | |
| | against rotation. Select two channels back to back. Assume that the column is | | |
| | laced and $fy = 250$ MPa. | | |
| 12. | Design a single lacing system for a column composed of 2 ISMC 300 @ 35.8 | 4, 5 | С |
| | kg/m placed back to back at clear spacing of 200mm. axial factored load on | | |
| | column is 1500 kN. Effective length of column is 5.0 m. | | |

TUTORIAL – 9 STEEL BEAM

| | | COs | LEVEL |
|---|--|------|-------|
| 1. Design a simply | y supported steel beam of 7 m spam carrying a RC floor | 4, 5 | С |
| capable of prov | capable of providing lateral restraint to the top compression flange. The | | |
| total factored uc | ll subjected was 53.6 kN/m throughout and factored point | | |
| load act at centre | e as 150 kN. Use ISMB section. Perform the check for web | | |
| buckling only. | | | |
| 2. A roof of a hal | I measuring 8m x 12m consist of 100mm thick R.C. slab | 4, 5 | С |
| supported on st | eel I-beams spaced 3 m apart. The finishing load may be | | |
| taken as 1.5kN/1 | m^2 and live load as 2 kN/m ² . Design the steel beam. | | |
| 3. Design a unifor | rm section for Moment and shear capacity of two spans | 4, 5 | С |
| simply supporte | ed continuous beam ABC. Span AB is of 4m length and | | |
| carries a central | concentrated load of 150kN and span BC is of 6m length | | |
| and carries a cer | ntral concentrated load of 200kN. Assume the beam is to be | | |
| laterally support | ed. Adopt plastic design procedure. | | |
| 4. Determine the | maximum uniformly distributed load that can be carried by | 3, 5 | E |
| a laterally unres | trained ISMB300 simply supported beam of 2.5m effective | | |
| length. | | | |
| 5. A simple suppo | ort beam is laterally supported over the span of 8m and | 4, 5 | С |
| loaded by a supe | er imposed load of 30kN/m over the entire span and 100kN | | |
| and centre. Des | ign the beam using ISMB section and check for all the | | |
| safety. | | | |
| 6. A beam of ISN | MB550 has simple support span of 9m and is laterally | 3, 5 | E |
| supported at cen | tre only. <u>Calculate</u> the maximum all inclusive factored udl | | |
| it can support. | | | |
| 7. Design an I sect | ion purlin for an industrial building to support a galvanized | 4, 5 | С |
| corrugated iron | sheet roof. Given data: | | |
| Spacing of the trusses = 5.0m, Spacing of purlins = 1.5 m | | | |
| Inclination of main | rafter to horizontal $= 30$ deg. | | |
| Weight of galvanize | ed sheets taking into account laps and connecting $bolt = 130$ | | |
| N/m ² | | | |
| Imposed snow load = 1.5 kN/m^2 . Wind load = 1.0 kN/m^2 | | | |
| 8. <u>Calculate</u> the m | noment carrying capacity of a 3 m long ISMB 350 beam | 3, 5 | E |
| which has full | torsional restraint and no warping restraint at ends only. | | |
| (Laterally unres | trained beam). | | |

TUTORIAL – 10

Slab Base and Gusseted Base

| | | COs | LEVEL |
|----|---|------|-------|
| 1. | Explain the design procedure of base plate. | 3, 4 | U |
| 2. | Explain the design procedure of gusseted base. | 3, 4 | U |
| 3. | Design a column base for a factored axial compressive load of 700KN and | 4, 5 | С |
| | a factored BM of 150KN-m about major axis. The column section provided | | |
| | is ISHB 400@ 806.4 N/m. Design the anchor bolts also, if required. The | | |
| | bearing pressure from concrete may be assumed to be 6.0 KN/m2. | | |
| 4. | Design a column cap for a truss transferring a reaction of 120 KN to a | 4, 5 | С |
| | column section ISHB 450 @ 907.43 N/m. | | |

PART C: TUTORIAL – 11 (RCC and Steel Structures)

TUTORIAL – 11

1. **Prepare** a sketch book with at least 20 sketches of different RCC and Steel members with detailing.