

Last Date for submission of Assignments

SUBJECT: Geotechnical Engineering

SUBJECT CODE: 3130606

SEMESTER: 3RD Civil Engineering

Assignment Number	Assignment Name	Last Date of Submission
1	Index Properties of Soil	13/10/2022
2	Permeability and Shear Strength of Soil	20/11/2022
3	Compaction and Consolidation of Soil	24/11/2022
4	Stress Distribution, Earth Pressure & Stability of Slopes	08/12/2022
5	Introduction to Foundations and Bearing Capacity	22/12/2022

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ASSIGNMENT 1: INDEX PROPERTIES OF SOIL

Sr. No.	Questions														
1	Explain soil formation in Geological cycle.														
2	What is weathering? Enumerate the type of weathering. Distinguish between physical and														
3	Write note on transportation of soil.														
4	Explain the types of soil in detail.														
5	Write note on soil types in Gujarat.														
6	Explain three phase diagrams of soil in detail.														
7	<p>Define following terms with respective equations.</p> <table style="width: 100%; border: none;"> <tr> <td style="width: 50%;">a) Water content</td> <td style="width: 50%;">b) Bulk unit weight</td> </tr> <tr> <td>c) Dry unit weight</td> <td>d) Saturated unit weight</td> </tr> <tr> <td>e) Submerged unit weight</td> <td>f) Unit weight of solids</td> </tr> <tr> <td>g) Specific Gravity</td> <td>h) Absolute specific gravity</td> </tr> <tr> <td>i) Void ratio</td> <td>j) Porosity</td> </tr> <tr> <td>k) Degree of saturation</td> <td>l) Air content</td> </tr> <tr> <td>m) Percentage air voids</td> <td>n) Relative density and give the range of relative density for different types of soil</td> </tr> </table>	a) Water content	b) Bulk unit weight	c) Dry unit weight	d) Saturated unit weight	e) Submerged unit weight	f) Unit weight of solids	g) Specific Gravity	h) Absolute specific gravity	i) Void ratio	j) Porosity	k) Degree of saturation	l) Air content	m) Percentage air voids	n) Relative density and give the range of relative density for different types of soil
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8	<p>Derive the following relationship.</p> <table style="width: 100%; border: none;"> <tr> <td style="width: 50%;">a) $e = \frac{n}{1-n}$</td> <td style="width: 50%;">b) $n = \frac{e}{1+e}$</td> </tr> <tr> <td>c) $\gamma_b = \frac{(G+e \cdot S_r)\gamma_w}{1+e}$</td> <td>d) $e = \frac{w \cdot G}{S_r}$</td> </tr> <tr> <td>e) $\gamma_d = \frac{\gamma_b}{1+w}$</td> <td></td> </tr> </table>	a) $e = \frac{n}{1-n}$	b) $n = \frac{e}{1+e}$	c) $\gamma_b = \frac{(G+e \cdot S_r)\gamma_w}{1+e}$	d) $e = \frac{w \cdot G}{S_r}$	e) $\gamma_d = \frac{\gamma_b}{1+w}$									
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9	Enlist the various methods of water content determination of soil and explain oven drying method in detail.
10	Explain specific gravity determination test.
11	Explain core cutter and sand replacement method.
12	A sample of silty clay has a volume of 14.88 cm ³ , total weight of 28.81 gm, dry weight is 24.83 gm and specific gravity is 2.7. Determine the void ratio and degree of saturation. <i>Answer: Void ratio= 0.618 and Degree of saturation= 70%</i>
13	A soil sample in its natural state has a weight of 2.29 kg and volume of 1.15x10 ⁻³ m ³ , under an oven dried state, the dry weight of the sample is 2.035 kg. The specific gravity of the soil is 2.68. Determine the total unit weight, water content, void ratio, porosity, degree of saturation and air content. <i>Answer: Total unit weight= 19.5 kN/m³, Water content= 12.5%, Void ratio= 0.51, porosity= 34%, Degree of saturation= 65.6%, Air Content= 12%</i>
14	A Soil sample has a porosity of 40 percent. The specific gravity of solids is 2.7. Calculate (a) Void ratio (b) Dry density (c) Unit weight if the soil is 50% saturated (d) Unit weight of soil if the soil is completely saturated. <i>Answer: (a) Void ratio = 0.67 (b) Dry density= 15.86 kN/m³ (c) unit weight at 50% saturation= 17.83 kN/m³ (d) unit weight for fully saturated soil= 19.79 kN/m³</i>
15	In a field density test, the volume and wet weight of soil obtained are 785 cc and 15.8 N respectively. If the water content is found to be 36%, determine the wet and dry unit weight of the soil. If the specific gravity of the soil grains is 2.6, compute the void ratio. <i>Answer: Wet unit weight= 20.11 kN/m³ , Dry unit weight= 14.78 kN/m³ and void ratio= 0.726</i>
16	Give difference between fine grained and coarse-grained soil.
17	Write short note on Particle size distribution curve.
18	Write short note on stokes law and also give assumptions and its limitations.
19	Draw Particle size distribution curves for different types of soils and also give its significance.
20	Describe the following terms with their probable range: Co-efficient of uniformity and Co-efficient of curvature.
21	What are different types of soil structure which can occur in nature? Describe in brief. Write short note on different shapes of the particles.
22	Write short note on diffuse double layer.
23	What is the purpose of Soil Classification?

24	Enlist various soil classification systems.												
25	<p>Write short note on followings</p> <table border="1" data-bbox="302 247 1300 338"> <tr> <td data-bbox="302 247 748 289">Particle size classification</td> <td data-bbox="748 247 1300 289">Textural classification</td> </tr> <tr> <td data-bbox="302 289 748 338">Unified soil classification</td> <td data-bbox="748 289 1300 338">IS Soil Classification</td> </tr> </table>	Particle size classification	Textural classification	Unified soil classification	IS Soil Classification								
Particle size classification	Textural classification												
Unified soil classification	IS Soil Classification												
26	Draw and explain IS Plasticity chart.												
27	Write short note on field identification of fine-grained soil.												
28	<p>Sieve analysis was conducted on a sample of coarse-grained soil and the following results obtained:</p> <p>Gravel= 12 %, Sand= 88 % D_{10}= 0.16 mm D_{30}= 0.64 mm D_{60}= 1.22 mm Classify the soil as per Unified soil Classification system.</p>												
29	<p>Define the following terms</p> <table data-bbox="302 905 976 1220"> <tr> <td data-bbox="302 905 732 947">Liquid limit</td> <td data-bbox="732 905 976 947">Plastic limit</td> </tr> <tr> <td data-bbox="302 957 732 999">Shrinkage limit</td> <td data-bbox="732 957 976 999">Plasticity index</td> </tr> <tr> <td data-bbox="302 1010 732 1052">Shrinkage index</td> <td data-bbox="732 1010 976 1052">Liquidity index</td> </tr> <tr> <td data-bbox="302 1062 732 1104">Flow index</td> <td data-bbox="732 1062 976 1104">Toughness index</td> </tr> <tr> <td data-bbox="302 1115 732 1157">Activity of Soil</td> <td data-bbox="732 1115 976 1157">Sensitivity of Soil</td> </tr> <tr> <td data-bbox="302 1167 732 1220">Thixotropy of Soil</td> <td></td> </tr> </table>	Liquid limit	Plastic limit	Shrinkage limit	Plasticity index	Shrinkage index	Liquidity index	Flow index	Toughness index	Activity of Soil	Sensitivity of Soil	Thixotropy of Soil	
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30	The liquid limit and plastic limit of a soil sample are 65 % and 29 % respectively. The percentage of soil fraction with grain size finer than 0.002 mm is 24. Calculate the activity ratio of the soil sample.												
31	The liquid limit of a soil is 62 % and its plastic limit is 34 %. Classify the soil as per IS Classification.												
32	<p>A test for the determination for the liquid limit gave the following observations. Plot the flow curve and determine liquid limit and flow index.</p> <table border="1" data-bbox="318 1619 1325 1717"> <tr> <td data-bbox="318 1619 813 1671">No. of Blows (N)</td> <td data-bbox="813 1619 935 1671">38</td> <td data-bbox="935 1619 1057 1671">27</td> <td data-bbox="1057 1619 1179 1671">20</td> <td data-bbox="1179 1619 1325 1671">13</td> </tr> <tr> <td data-bbox="318 1671 813 1717">Water Content %</td> <td data-bbox="813 1671 935 1717">47</td> <td data-bbox="935 1671 1057 1717">49</td> <td data-bbox="1057 1671 1179 1717">51</td> <td data-bbox="1179 1671 1325 1717">53</td> </tr> </table>	No. of Blows (N)	38	27	20	13	Water Content %	47	49	51	53		
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ASSIGNMENT 2: PERMEABILITY AND SHEAR STRENGTH OF SOIL

1. Write short note on soil water present in soil mass.
2. Define total stress, neutral stress and effective stress. What is the importance of the effective stress?
3. A deposit of sand 10m thick overlies a bed of soft clay. The ground water table is 3m below the ground surface. If the sand above the ground water table has a degree of saturation 40%, plot the diagram showing the variation of the total stress, neutral stress and effective stress. Take $G= 2.65$ and $e=0.70$.
4. Define Following terms
 - a) Permeability
 - b) Total Head
 - c) Hydraulic Head
 - d) Hydraulic Gradient
 - e) Percolation (Seepage)
 - f) Discharge velocity
 - g) Seepage Velocity
 - h) Piping
 - i) Boiling or Quick Sand
 - j) Coefficient of permeability
 - k) Aquifer
 - l) Aquiclude
 - m) Aquitard
 - n) Aquifuge
 - o) Unconfined aquifer
 - p) Confined aquifer
5. What is flow net and characteristics of flow net? Also explain the application of flow net?
6. A horizontal stratified deposit of three layer of soil having of 3 m, 2 m and 4 m respectively. The coefficient of permeability of corresponding individual layer is 3×10^{-3} cm/s, 6.5×10^{-2} cm/s and 7×10^{-4} cm/s. Calculate the effective equivalent coefficient of permeability of the deposit in (a) Horizontal direction (b) Vertical direction
7. What is the importance of permeability in the field? Enlist the laboratory and field test of permeability and explain Constant head permeability test.
8. How will you get shear strength parameters of soil? Describe direct box shear test. What are its limitations?

9. Explain Mohr-coulomb's Strength theory. Sketch typical Mohr-coulomb's strength envelop for C- Soil ϕ - Soil and C- ϕ Soil.
10. Briefly explain the shear tests which may be performed based on the different drainage conditions? With its field applications.
11. What are the advantages and disadvantages of Triaxial Shear Test.?
12. Describe unconfined compression test. What are its advantages over Triaxial test?
13. Describe laboratory vane shear test with neat sketch.
14. In an unconfined compression test, a sample of clay 8 cm long and 4 cm in diameter fail under a load of 120 N at 10% strain. Find the shearing resistance taking into account the effect of change in cross section of sample.
15. Determine the shearing strength parameters from the Direct Shear Test results given below:

Sr. No.	Normal Stress (kN/m ²)	Shear Stress (kN/m ²)
1.	150	110
2.	200	120

What would be the deviator stress if triaxial test is carried out on same soil with cell pressure of 150 kN/m²?

16. From the Undrained Triaxial test results given below, determine the shear strength parameters c & ϕ .

Sr. No.	Cell Pressure (kPa)	Deviator Stress (kPa)
1.	200	690
2.	400	840
3.	600	990

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ASSIGNMENT 3: COMPACTION AND CONSOLIDATION OF SOIL

1. Explain factors affecting compaction of soil.
2. Write Difference between heavy compaction test and light compaction test.
3. Discuss the effects of compaction on various soil properties
4. The maximum dry density and optimum moisture content of soil from standard proctor test are 1.9 kN/m³ and 14% respectively. Compute water content necessary to completely saturate the sample at its maximum dry density. Assuming no change in volume $G = 2.7$
5. The following data were recorded while performing the compaction test: -

Water content (%)	7.71	11.5	14.6	17.50	19.50	21.25
Bulk density (kN/m ³)	17.55	19.50	21.0	20.55	20.30	19.80

Plot the MDD-OMC curve and obtain the optimum water content and maximum dry density. Also plot zero air voids curve. Take $G = 2.66$

6. The following are the observation of a compaction test.

Water content (w %)	7.7	11.5	14.6	17.5	19.5	21.2
Wt. of wet soil W (N)	16.67	18.54	19.92	19.52	19.23	18.83

the volume of compaction mould is 950 cc. Assuming $G = 2.65$. Draw the compaction curve. Report maximum dry unit weight and optimum moisture content (OMC). Draw 100% saturation line (zero air void line). What is the degree of saturation at OMC?

7. Explain theory of spring analogy for primary consolidation.
8. Differentiate between normally consolidated and over-consolidated soils.
9. Enlist various causes for Pre-consolidation of soils. Discuss the Casagrande's method to determine the pre-consolidation pressure from consolidation test data.
10. Explain 'Square root of time fitting method' and 'logarithm of time fitting method' for determination of coefficient of consolidation.

11. During consolidation test, the void ratio decreases from 0.80 to 0.50 under the stress increment of 2.0 kg/cm² to 4.0 kg/cm². Compute coefficient of compressibility & coefficient of volume compressibility.

12. Define the following terms:

- Primary Consolidation
- Secondary Consolidation
- Coefficient of Compressibility,
- Coefficient of Volume Compressibility,
- Compression Index
- Coefficient of Consolidation
- Degree of Consolidation
- Cohesion (C)
- Angle of Internal Friction (ϕ)

13. Explain 'Square root of time fitting method' and 'logarithm of time fitting method' for determination of coefficient of consolidation.

14. During consolidation test, the void ratio decreases from 0.80 to 0.50 under the stress increment of 2.0 kg/cm² to 4.0 kg/cm². Compute coefficient of compressibility & coefficient of volume compressibility.

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ASSIGNMENT 4: STRESS DISTRIBUTION, EARTH PRESSURE & STABILITY OF SLOPES

1.	Write short note on “New mark’s influence chart”.
2.	Derive the “Boussinesq’s equation of vertical stress and tangential stress subjected to concentrated force.
3.	Illustrate the procedure for plotting an isobar of intensity 0.1 Q. Also draw Isobar diagram.
4.	What is pressure bulb? Explain its use.
5.	A concentrated load of 50 kN acts on the surface of a homogeneous soil mass of large extent. Find the stress intensity at a depth of 5m and Directly under the load, and at a horizontal distance of 5m. Use Boussinesq’s equation.
6.	A rectangular foundation 4m x 3m transmits a uniformly pressure of 450 kN/m ² to the under-laying soil. Determine the Vertical stress at a point 3 m vertically below a point lying within the loaded area 1.5 m away from the short edge and 1 m away from the long edge. Use Boussinesq’s theory.
7.	What are the graphical methods available for the determination of lateral earth pressure? Explain any one in detail.
8.	Elaborates the shortcomings of Rankine’s active earth pressure theory and Coulomb’s active earth pressure theory.
9.	Write a short note on “Active Earth Pressure for Cohesive Soils”.
10.	What are different types of Earth Pressures? Give examples.
11.	Define Earth pressure at Rest. Show the earth pressure distribution on a retaining wall, assuming that the soil fill is dry.
12.	A retaining wall of 4 m high has a smooth vertical back. The backfill has a level with the top of the wall. There is a uniformly distributed surcharge load of 36 kN/m ² , intensity over backfill. The unit weight of the backfill is 18 kN/m ³ , its angle of shearing resistance is 30 ⁰ and cohesion is zero. Determine the magnitude and point of application of active earth pressure per meter length of the soil.

13.	<p>Determine the active earth pressure on the retaining wall of 5 m high for following data.</p> <ol style="list-style-type: none"> 1. Top 2.5 m, soil $\phi = 35^\circ$ and $\gamma = 17 \text{ kN/m}^3$ 2. Bottom 2.5 m, soil $\phi = 38^\circ$ and $\gamma = 18 \text{ kN/m}^3$ <p>Water level is at 2.5 m below ground surface. Adopt $\gamma_w = 10 \text{ kN/m}^3$.</p>
14.	<p>A retaining wall, 5 m. high, with vertical back, retains soil fill, the upper face of fill is horizontal. The back face of the wall is smooth. Determine the Rankine active earth pressure on the wall, (a) before the formation of crack and (b) after the formation of crack. Take $C = 5 \text{ kN/m}^2$, $\gamma = 17.5 \text{ kN/m}^3$, and Angle of internal friction = 30°.</p>
15.	<p>A retaining wall, 6 m. high, retains dry sand with an angle of friction of 30°, and unit weight of 16.2 kN/m^3. Determine the earth pressure at rest. If the water table rises to the top of the wall, determine the increase in the thrust on the wall. Assume the submerged unit weight of sand as 10 kN/m^3.</p>
16.	<p>What are different factors of safety used in the stability of slopes? Discuss briefly.</p>
17.	<p>Discuss briefly, different types of slope failures.</p>
18.	<p>How a slope is analyzed using a Swedish circle method (method of slices)? Discuss the method and derive an expression for the factor of safety.</p>
19.	<p>Briefly describe the method of slices for finite slope stability analysis for C-ϕ soil.</p>
20.	<p>A long natural slope in C-ϕ soil is inclined at 12° to the horizontal. The water table is at the surface and seepage is parallel to the slope. If a plane slip has developed at a depth of 4.0 m, determine the factor of safety. Take $C = 10 \text{ kPa}$, $\phi = 20^\circ$ & $\gamma_{\text{sat}} = 20 \text{ kN/m}^3$.</p>
21.	<p>Determine the factor of safety against sliding for a slip surface passing through the toe of a finite slope of height of 11m and slope angle of 1V:1.5H has $c = 15 \text{ kPa}$, $\phi = 32^\circ$ and $\gamma = 20 \text{ kN/m}^3$. The radius & the central angle of the slip circle are 17.4 m & 870 respectively. Take $\Sigma N = 1902.74 \text{ kN}$ and $\Sigma T = 941.15 \text{ kN}$. Use Swedish slip circle method.</p>

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ASSIGNMENT 5: INTRODUCTION TO FOUNDATIONS AND BEARING CAPACITY

1. Define the followings terms.
 - a) Foundation
 - b) Footing
 - c) Ultimate Bearing Capacity
 - d) Net Ultimate Bearing Capacity
 - e) Net Safe Bearing Capacity
 - f) Gross Safe Bearing Capacity
 - g) Net Safe settlement Pressure
 - h) Net Allowable Bearing Pressure
2. Write a note on Terzaghi's bearing capacity theory. What are the assumptions made in Terzaghi's bearing capacity theory?
3. Explain the factors effecting the selection of types of foundation.
4. State different types of shallow foundation.
5. Differentiate between shallow foundation and deep foundation.
6. Draw neat sketch of followings (i) Raft Foundation (ii) well foundation (iii) cofferdams (iv) Strip footing (v) strap footing (vi) spread footing (vii) R.C.C. footing
7. Enumerate the factors affecting bearing capacity and explain any two in detail.
8. Discuss bearing capacity from standard penetration test.
9. Discuss bearing capacity Based on IS- Code Method.
10. Determine the ultimate bearing capacity of a strip footing, 1.2 m wide and having the depth of foundation

of 1 m. use Terzaghi's theory and assume general shear failure, take $\phi' = 35^\circ$, $\gamma = 18 \text{ kN/m}^3$, and $c' = 15 \text{ kN/m}^2$.

Ans. 2070 kN/m².

11. A footing 2 m square is laid at a depth of 1.3 m below the ground surface. Determine the net ultimate bearing capacity using IS Code method. take $\phi' = 30^\circ$, $\gamma = 20 \text{ kN/m}^3$, and $c' = 0 \text{ kN/m}^2$.

Ans. $q_{nu} = 1000 \text{ kN/m}^2$

12. A strip footing of 2 m width is founded at a depth of 4 m below the ground surface. Determine the net ultimate bearing capacity using (a) Terzaghi's equation (b) Kempton's equation and (c) IS code method. The unit weight of the soil is 20 kN/m^3 .

Ans. (a) Terzaghi's $q_{nu} = 57 \text{ kN/m}^2$ (b) Kempton's $q_{nu} = 70 \text{ kN/m}^2$ (c) $q_{nu} = 71.96 \text{ kN/m}^2$

13. Classify the different types of pile.

14. Differentiate between pile foundation and shallow foundation.

15. What is negative skin friction? What is its effect on pile?

16. Discuss various dynamic formulas. What are their limitations?

17. How to estimate the load carrying capacity of a pile in Cohesionless and cohesive soil?

18. How would you estimate the group capacity of piles in sand and clay?

19. Write a note on under reamed pile and also give codal provisions for under reamed pile.

20. Give methods to determine the pile capacity and explain any two.

21. Write a note on group action and efficiency of pile group.

22. A square concrete pile 30 cm x 30 cm is driven in to homogeneous sand layer, ($\phi' = 30^\circ$, $\gamma = 18 \text{ kN/m}^3$) for a depth of 12 m. calculate the ultimate load. Take $K = 1.3$, $\delta = 18^\circ$.

Ans: $Q_u = 611.81 \text{ kN}$.

23. A group of friction pile in clay consists of 15 piles of 500 mm diameter grouped as 5x3 spaced at 1m apart. If the undrained shear strength of clay is 0.3 N/cm^2 and piles are 20 m long, estimate the group capacity and its efficiency.

Ans. Pile group capacity = 1143.75 kN and Pile group efficiency = 56.70%